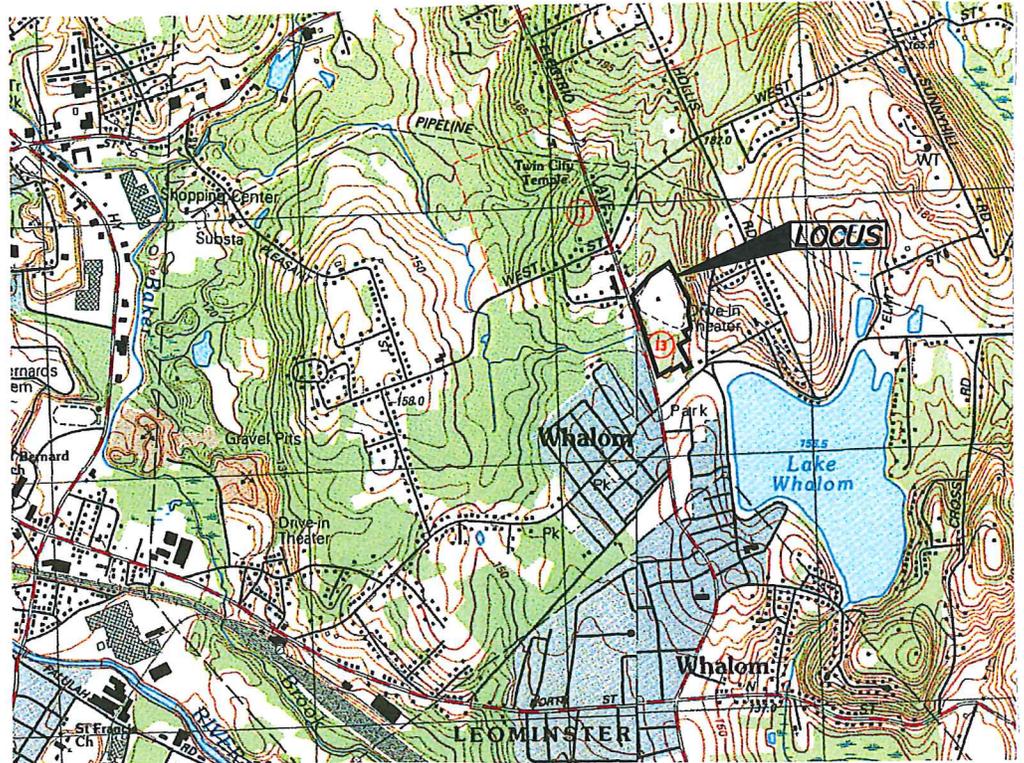


STORMWATER REPORT AND DRAINAGE CALCULATIONS

Pursuant to
DEPARTMENT OF ENVIRONMENTAL PROTECTION
Stormwater Rules and Regulations

Proposed Unitil Facility
357 Electric Avenue
Lunenburg, MA



November 20, 2015

Prepared for:
Unitil
306 Highlander Way
Manchester, NH 03103

Prepared by:
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WHITMAN & BINGHAM
ASSOCIATES
REGISTERED ENGINEERS & LAND SURVEYORS

STORMWATER CHECKLIST



Checklist for Stormwater Report

A. Introduction

Important:

When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the [Massachusetts Stormwater Handbook](#). The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.¹ This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8²
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

¹ The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

² For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.



Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

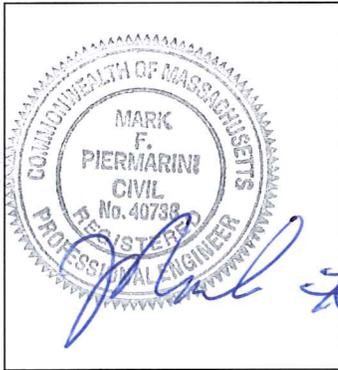
Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature



Mark F. Piermarini 11-20-15

Signature and Date

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

- New development
- Redevelopment
- Mix of New Development and Redevelopment



Checklist for Stormwater Report

Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

- No disturbance to any Wetland Resource Areas
- Site Design Practices (e.g. clustered development, reduced frontage setbacks)
- Reduced Impervious Area (Redevelopment Only)
- Minimizing disturbance to existing trees and shrubs
- LID Site Design Credit Requested:
 - Credit 1
 - Credit 2
 - Credit 3
- Use of "country drainage" versus curb and gutter conveyance and pipe
- Bioretention Cells (includes Rain Gardens)
- Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
- Treebox Filter
- Water Quality Swale
- Grass Channel
- Green Roof
- Other (describe): _____

Standard 1: No New Untreated Discharges

- No new untreated discharges
- Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



Checklist for Stormwater Report

Checklist (continued)

Standard 2: Peak Rate Attenuation

- Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.
- Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

Standard 3: Recharge

- Soil Analysis provided.
- Required Recharge Volume calculation provided. *- REDUCED IMPERVIOUS AREA BY 1.55 ACRES THEREFORE INCREASE IN RECHARGE OF SITE.*
- Required Recharge volume reduced through use of the LID site Design Credits.
- Sizing the infiltration, BMPs is based on the following method: Check the method used.
 - Static
 - Simple Dynamic
 - Dynamic Field¹
- Runoff from all impervious areas at the site discharging to the infiltration BMP.
- Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:
 - Site is comprised solely of C and D soils and/or bedrock at the land surface
 - M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
 - Solid Waste Landfill pursuant to 310 CMR 19.000
 - Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- Calculations showing that the infiltration BMPs will drain in 72 hours are provided.
- Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

¹ 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



Checklist for Stormwater Report

Checklist (continued)

Standard 3: Recharge (continued)

- The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
 - Provisions for storing materials and waste products inside or under cover;
 - Vehicle washing controls;
 - Requirements for routine inspections and maintenance of stormwater BMPs;
 - Spill prevention and response plans;
 - Provisions for maintenance of lawns, gardens, and other landscaped areas;
 - Requirements for storage and use of fertilizers, herbicides, and pesticides;
 - Pet waste management provisions;
 - Provisions for operation and management of septic systems;
 - Provisions for solid waste management;
 - Snow disposal and plowing plans relative to Wetland Resource Areas;
 - Winter Road Salt and/or Sand Use and Storage restrictions;
 - Street sweeping schedules;
 - Provisions for prevention of illicit discharges to the stormwater management system;
 - Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
 - Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
 - List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
 - Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
 - is within the Zone II or Interim Wellhead Protection Area
 - is near or to other critical areas
 - is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
 - involves runoff from land uses with higher potential pollutant loads.
 - The Required Water Quality Volume is reduced through use of the LID site Design Credits.
 - Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.

- RE-Development - TSS REMOVAL TO MAXIMUM EXTENT PRACTICABLE.



Checklist for Stormwater Report

Checklist (continued)

Standard 4: Water Quality (continued)

- The BMP is sized (and calculations provided) based on:
 - The ½" or 1" Water Quality Volume or
 - The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
- The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the proprietary BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
- A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

N/A

Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

- The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.
- The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted *prior to* the discharge of stormwater to the post-construction stormwater BMPs.
- The NPDES Multi-Sector General Permit does *not* cover the land use.
- LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
- All exposure has been eliminated.
- All exposure has *not* been eliminated and all BMPs selected are on MassDEP LUHPPL list.
- The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

N/A

Standard 6: Critical Areas

- The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
- Critical areas and BMPs are identified in the Stormwater Report.



Checklist for Stormwater Report

Checklist (continued)

Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

- The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:
- Limited Project
 - Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
 - Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
 - Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
 - Bike Path and/or Foot Path
- Redevelopment Project
- Redevelopment portion of mix of new and redevelopment.
- Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.
- The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
 - Construction Period Operation and Maintenance Plan;
 - Names of Persons or Entity Responsible for Plan Compliance;
 - Construction Period Pollution Prevention Measures;
 - Erosion and Sedimentation Control Plan Drawings;
 - Detail drawings and specifications for erosion control BMPs, including sizing calculations;
 - Vegetation Planning;
 - Site Development Plan;
 - Construction Sequencing Plan;
 - Sequencing of Erosion and Sedimentation Controls;
 - Operation and Maintenance of Erosion and Sedimentation Controls;
 - Inspection Schedule;
 - Maintenance Schedule;
 - Inspection and Maintenance Log Form.
- A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



Checklist for Stormwater Report

Checklist (continued)

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

- The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has **not** been included in the Stormwater Report but will be submitted **before** land disturbance begins.
- The project is **not** covered by a NPDES Construction General Permit.
- The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

- The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
 - Name of the stormwater management system owners;
 - Party responsible for operation and maintenance;
 - Schedule for implementation of routine and non-routine maintenance tasks;
 - Plan showing the location of all stormwater BMPs maintenance access areas;
 - Description and delineation of public safety features;
 - Estimated operation and maintenance budget; and
 - Operation and Maintenance Log Form.
- The responsible party is **not** the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
 - A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
 - A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

- The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- An Illicit Discharge Compliance Statement is attached;
- NO Illicit Discharge Compliance Statement is attached but will be submitted **prior to** the discharge of any stormwater to post-construction BMPs.

STORMWATER REPORT



510 Mechanic Street
Leominster, Massachusetts 01453
(978) 537-5296
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STORMWATER REPORT
PROPOSED UNITIL FACILITY
357 ELECTRIC AVENUE

Lunenburg, MA

November 20, 2015

**Stormwater Report –
Proposed Unutil Facility, 357 Electric Avenue, Lunenburg, MA**

Introduction

This Stormwater Report has been prepared in accordance with the Massachusetts Department of Environmental Protection (DEP) Stormwater Management Standards and the Stormwater Management Handbook.

Project Type

The site is approximately 15.63 acres located at 357 Electric Avenue in Lunenburg, MA. The existing site was formerly a Drive-In Theatre with approximately 6.81 acres of impervious pavement. The proposed project will be the site of a Unutil – Fitchburg Gas and Electric Light Company Facility with a 45,000 S.F. building and associated driveways, parking areas and additional facility areas as shown on the site drawings. The project will be designed as a re-development.

LID Measures

The LID measures considered are reduced impervious area (re-development), minimizing disturbance to existing trees along the perimeter of the property, no disturbance to wetland resource areas, “country” drainage design and grass channels.

Standard 1: No New Untreated Discharges

The project is designed so that there are no new stormwater point discharges that discharge untreated stormwater into, or cause erosion to, wetlands and waters of the Commonwealth of Massachusetts. The new site drainage system will include a catch basin, vegetated filter strip, stone check dams and grass channels.

Standard 2: Peak Rate Attenuation:

The project drainage system was designed to attenuate the peak discharge rates of runoff from the site. As stated in the “Drainage Calculations”, through the re-development of the site the runoff rates for the post-development conditions can be effectively maintained at or below pre-development rates at the design point utilized in the calculations. The calculations considered the 2, 10, 25 and 100 year, 24 hour storm events. The calculations were completed using NRCS Technical Release 55.

Standard 3: Recharge

The site is located in an area of Urban Land Soils. The Urban Land soils were assumed as a C soil type. The proposed re-development of the site, reducing the impervious area from 6.81 acres to 5.26 acres, will provide an increase of approximately 1,406 C.F. for recharge of runoff (as shown in the attached Recharge Volume Calculations within the drainage calculations). Therefore, standard 3 has been met.

Standard 4: Water Quality

Through the use of the proposed parking area sweeping, vegetated filter strip, stone check dams and grass channels the stormwater management system is expected to remove approximately 55% of total suspended solids (TSS) from runoff collected by the system (as shown in the attached TSS Removal Worksheet within the drainage calculations). Therefore, standard 4 has been met to the maximum extent practicable.

Standard 5: Land Uses with Higher Potential Pollutant Loads (LUHPPLs)

The project area does not contain a land use with higher potential pollutant loads.

Standard 6: Critical Areas

The stormwater management system proposed for the site does not discharge within a Zone II or Interim Wellhead Protection Area of a public water supply.

Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

The proposed project is a redevelopment project and is subject to the Stormwater Management Standards only to the Maximum Extent Practicable.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

The site plans for the proposed residential project indicate the proposed locations of erosion control barriers and other information as required addressing the construction period erosion and sediment control. Additional information regarding the construction period protection, inspection, and maintenance of the installed stormwater management system can be found in the attached document "Stormwater Management System Inspection and Maintenance Plan – Proposed Unitil Facility, 357 Electric Avenue, Lunenburg, MA" dated November 20, 2015.

The construction on and off-site will result in the disturbance of more than one acre of land therefore an NPDES Permit including a SWPPP will be required for the project and will be submitted prior to the start of construction.

Standard 9: Operation and Maintenance Plan

Items required under Standard 9 are provided in the attached document “Stormwater Management System Inspection and Maintenance Plan – Proposed Unitil Facility, 357 Electric Avenue, Lunenburg, MA” dated November 20, 2015. This document includes information pertaining to the protection, inspection, and maintenance of the installed stormwater management system both during and after the construction period.

Standard 10: Prohibition of Illicit Discharges

The proposed Unitil Facility site does not include any illicit discharges of stormwater or other source of illicit discharge. No use of the site will include discharges to the stormwater management system that include any wastewater discharges or discharges of stormwater contaminated by contact with process wastes, raw materials, toxic pollutants, hazardous substances, oil, or grease.

The project site plans show the locations of all components of the stormwater management system.

Registered Professional Engineers Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.



[Handwritten Signature]

Signature

11-20-15

Date

DRAINAGE CALCULATIONS

INTRODUCTION

The overland flow drainage calculations were performed using the SCS method and TR-55.

The site is approximately 15.63 acres located at 357 Electric Avenue in Lunenburg, MA. The existing site was formerly a Drive-In Theatre with approximately 6.81 acres of impervious pavement. The project will be the site of a proposed Unitil – Fitchburg Gas and Electric Light Company Facility with a 45,000 S.F. building and associated driveways, parking areas and facility areas as shown on the site drawings. The overall project area consists of gentle to moderate slopes which start from the northeasterly corner of the site and drain toward the southerly end of the site.

The drainage system for the site will consist of a catch basin, vegetated filter strip, stone check dams and grass channels. The proposed site re-development has been designed so there is a total of 1.55 acres of impervious area that shall be removed, therefore the site will retain more stormwater runoff post-development compared to pre-development and meet standard 3 of the stormwater management regulations.

The drainage system has been designed in accordance with the Massachusetts DEP Storm Water Quality Standards. The following Best Management Practices (BMP) have been used on the site: catch basin with deep sump, vegetated filter strip, stone check dams and grass channels.

OBJECTIVE

Runoff rates for the watersheds listed in the following tables have been calculated for the 2, 10, 25 and 100-year storms. The proposed site re-development has been designed to maintain pre-development rates of runoff at or below the post-development conditions.

DRAINAGE SUMMARY:

Tables I and II below are a summary of pre-development and post-development peak runoff rates for overland flow toward the southerly design point. (please refer to the “Pre- & Post-Development subcatchment” maps for delineation of these areas).

TABLE I

PRE-DEVELOPMENT CONDITIONS				
<i>WATERSHED</i>	<i>FLOW (CFS)</i>			
	2 year	10 year	25 year	100 year
1S	15.42	24.76	29.71	37.68

TABLE II

POST-DEVELOPMENT CONDITIONS					
<i>WATERSHED</i>	<i>COMPARED TO PRE- DEVELOPMENT WATERSHED</i>	<i>FLOW (CFS)</i>			
		2 year	10 year	25 year	100 year
DP	(1S)	12.21 (15.42)	20.99 (24.76)	25.70 (29.71)	33.27 (37.68)

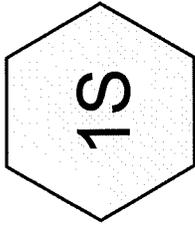
(CFS) - CUBIC FEET PER SECOND

CONCLUSION

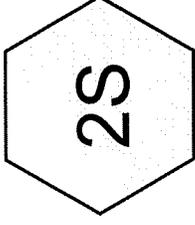
Through the proposed re-development of the site the runoff rates for the post-development condition can be effectively maintained at or below the pre-development runoff rates.

EXISTING CONDITIONS

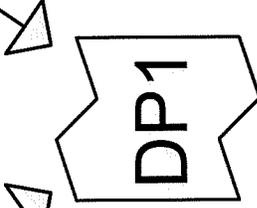
2 YEAR STORM



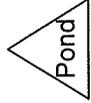
Overland runoff to southwest



Overland runoff to south



Design Point 1



357 Electric Avenue - Pre-development

Prepared by {enter your company name here}

HydroCAD® 7.10 s/n 000655 © 2005 HydroCAD Software Solutions LLC

2 Year Storm
Type III 24-hr Rainfall=3.00"

Page 1

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Overland runoff to southwest

Runoff Area=5.450 ac Runoff Depth>2.21"
Flow Length=708' Tc=17.5 min CN=94 Runoff=10.30 cfs 1.006 af

Subcatchment 2S: Overland runoff to south

Runoff Area=2.870 ac Runoff Depth>1.94"
Flow Length=498' Tc=15.6 min CN=91 Runoff=5.13 cfs 0.465 af

Link DP1: Design Point 1

Inflow=15.42 cfs 1.471 af
Primary=15.42 cfs 1.471 af

Total Runoff Area = 8.320 ac Runoff Volume = 1.471 af Average Runoff Depth = 2.12"

357 Electric Avenue - Pre-development

Prepared by {enter your company name here}

HydroCAD® 7.10 s/n 000655 © 2005 HydroCAD Software Solutions LLC

2 Year Storm
Type III 24-hr Rainfall=3.00"

Page 2

11/20/2015

Subcatchment 1S: Overland runoff to southwest

Runoff = 10.30 cfs @ 12.23 hrs, Volume= 1.006 af, Depth> 2.21"

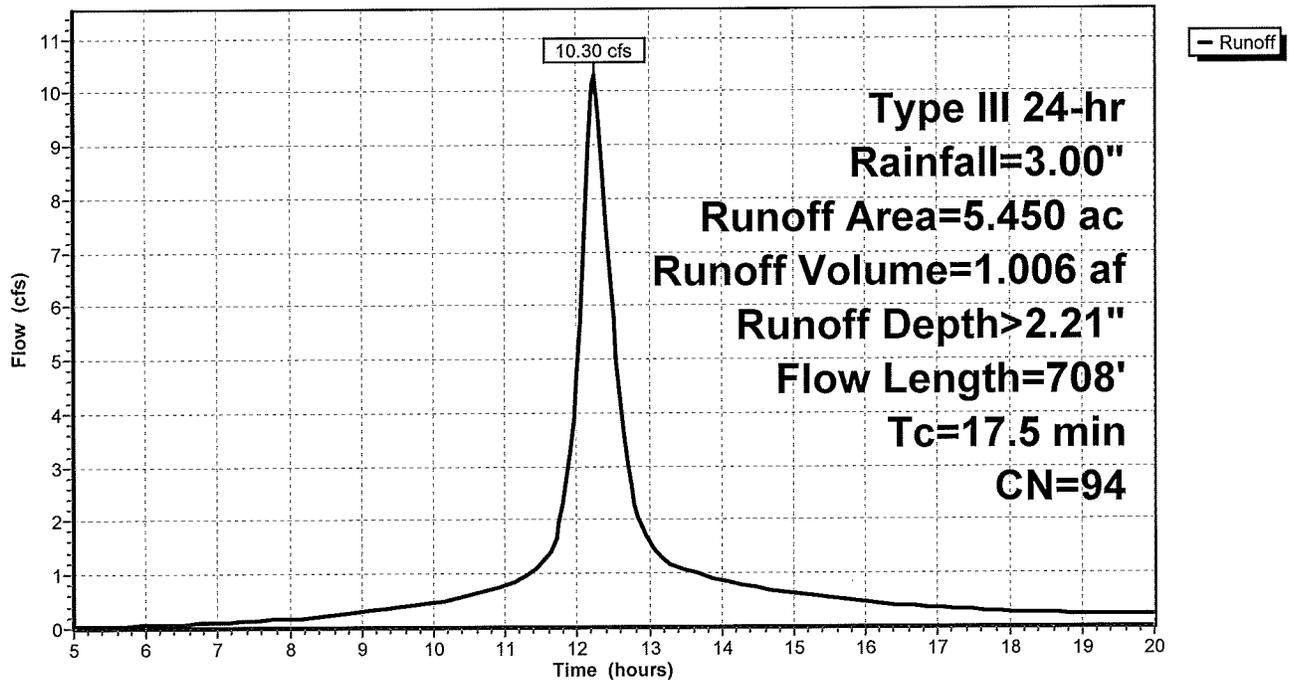
Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr Rainfall=3.00"

Area (ac)	CN	Description
4.610	98	Paved parking & roofs
0.840	70	Woods, Good, HSG C
5.450	94	Weighted Average

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.7	50	0.0200	0.1		Sheet Flow, Path 1 Woods: Light underbrush n= 0.400 P2= 3.00"
3.4	588	0.0200	2.9		Shallow Concentrated Flow, Path 2 Paved Kv= 20.3 fps
1.4	70	0.0285	0.8		Shallow Concentrated Flow, Path 3 Woodland Kv= 5.0 fps
17.5	708	Total			

Subcatchment 1S: Overland runoff to southwest

Hydrograph



357 Electric Avenue - Pre-development

Prepared by {enter your company name here}

HydroCAD® 7.10 s/n 000655 © 2005 HydroCAD Software Solutions LLC

2 Year Storm
Type III 24-hr Rainfall=3.00"

Page 3

11/20/2015

Hydrograph for Subcatchment 1S: Overland runoff to southwest

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)	Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
5.00	0.17	0.00	0.02	17.75	2.77	2.13	0.29
5.25	0.18	0.00	0.03	18.00	2.78	2.14	0.28
5.50	0.19	0.01	0.04	18.25	2.80	2.15	0.26
5.75	0.20	0.01	0.04	18.50	2.81	2.16	0.25
6.00	0.22	0.01	0.05	18.75	2.82	2.18	0.24
6.25	0.23	0.01	0.06	19.00	2.83	2.19	0.24
6.50	0.24	0.02	0.07	19.25	2.84	2.20	0.23
6.75	0.26	0.02	0.08	19.50	2.85	2.21	0.22
7.00	0.27	0.03	0.10	19.75	2.86	2.22	0.22
7.25	0.29	0.03	0.11	20.00	2.87	2.23	0.21
7.50	0.30	0.04	0.13				
7.75	0.32	0.05	0.15				
8.00	0.34	0.05	0.17				
8.25	0.36	0.06	0.19				
8.50	0.39	0.07	0.22				
8.75	0.41	0.09	0.26				
9.00	0.44	0.10	0.29				
9.25	0.47	0.12	0.33				
9.50	0.50	0.14	0.38				
9.75	0.53	0.16	0.42				
10.00	0.57	0.18	0.47				
10.25	0.61	0.20	0.53				
10.50	0.65	0.23	0.60				
10.75	0.70	0.27	0.69				
11.00	0.75	0.31	0.79				
11.25	0.81	0.36	0.92				
11.50	0.89	0.42	1.20				
11.75	1.07	0.56	1.93				
12.00	1.50	0.94	4.62				
12.25	1.93	1.33	10.26				
12.50	2.11	1.50	5.79				
12.75	2.19	1.57	2.63				
13.00	2.25	1.63	1.60				
13.25	2.30	1.68	1.21				
13.50	2.35	1.73	1.06				
13.75	2.39	1.77	0.96				
14.00	2.43	1.81	0.87				
14.25	2.47	1.84	0.78				
14.50	2.50	1.87	0.73				
14.75	2.53	1.90	0.68				
15.00	2.56	1.93	0.64				
15.25	2.59	1.96	0.60				
15.50	2.61	1.98	0.55				
15.75	2.64	2.00	0.51				
16.00	2.66	2.02	0.46				
16.25	2.68	2.04	0.42				
16.50	2.70	2.06	0.39				
16.75	2.71	2.07	0.37				
17.00	2.73	2.09	0.35				
17.25	2.74	2.10	0.33				
17.50	2.76	2.12	0.31				

357 Electric Avenue - Pre-development

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2 Year Storm
Type III 24-hr Rainfall=3.00"

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Subcatchment 2S: Overland runoff to south

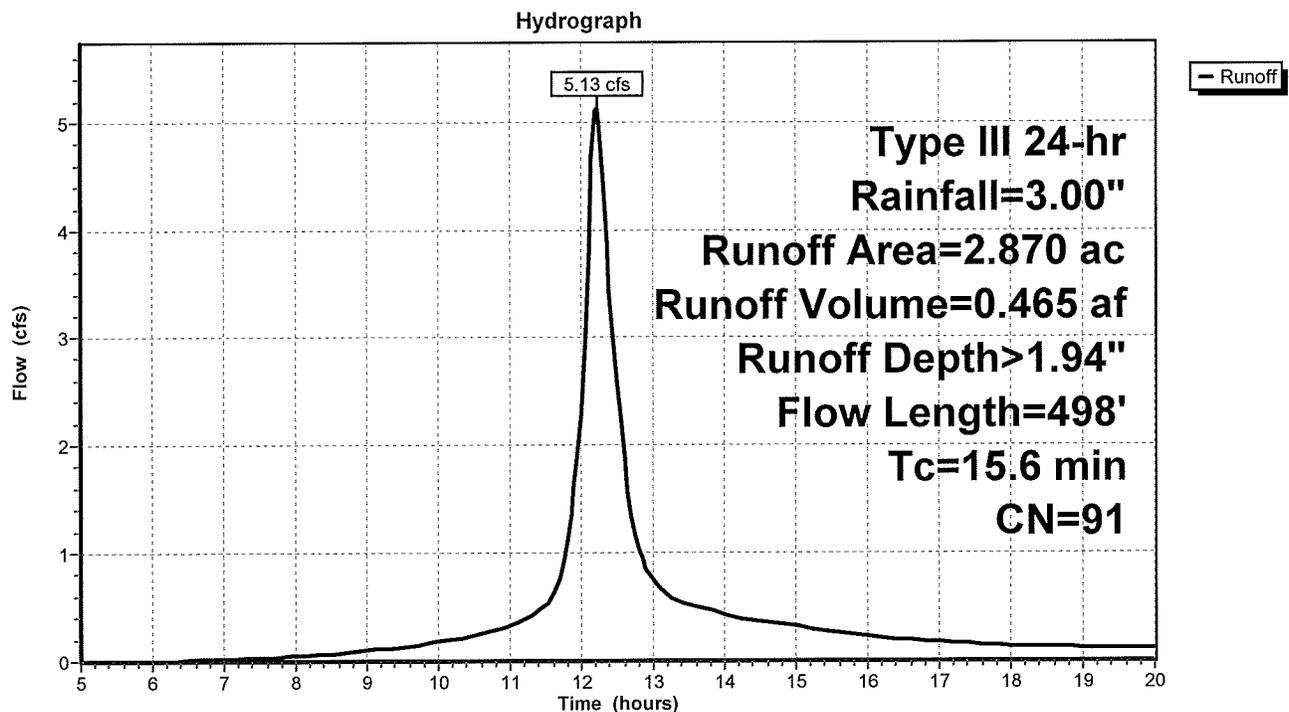
Runoff = 5.13 cfs @ 12.21 hrs, Volume= 0.465 af, Depth> 1.94"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr Rainfall=3.00"

Area (ac)	CN	Description
2.200	98	Paved parking & roofs
0.670	70	Woods, Good, HSG C
2.870	91	Weighted Average

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.7	50	0.0200	0.1		Sheet Flow, Path 1 Woods: Light underbrush n= 0.400 P2= 3.00"
2.3	408	0.0220	3.0		Shallow Concentrated Flow, Path 2 Paved Kv= 20.3 fps
0.6	40	0.0500	1.1		Shallow Concentrated Flow, Path 3 Woodland Kv= 5.0 fps
15.6	498	Total			

Subcatchment 2S: Overland runoff to south



357 Electric Avenue - Pre-development

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2 Year Storm
Type III 24-hr Rainfall=3.00"

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Hydrograph for Subcatchment 2S: Overland runoff to south

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)	Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
5.00	0.17	0.00	0.00	17.75	2.77	1.86	0.15
5.25	0.18	0.00	0.00	18.00	2.78	1.87	0.14
5.50	0.19	0.00	0.00	18.25	2.80	1.88	0.13
5.75	0.20	0.00	0.00	18.50	2.81	1.89	0.13
6.00	0.22	0.00	0.00	18.75	2.82	1.90	0.12
6.25	0.23	0.00	0.01	19.00	2.83	1.91	0.12
6.50	0.24	0.00	0.01	19.25	2.84	1.92	0.12
6.75	0.26	0.00	0.01	19.50	2.85	1.93	0.11
7.00	0.27	0.01	0.02	19.75	2.86	1.94	0.11
7.25	0.29	0.01	0.02	20.00	2.87	1.95	0.11
7.50	0.30	0.01	0.03				
7.75	0.32	0.01	0.04				
8.00	0.34	0.02	0.05				
8.25	0.36	0.02	0.05				
8.50	0.39	0.03	0.07				
8.75	0.41	0.04	0.08				
9.00	0.44	0.05	0.10				
9.25	0.47	0.06	0.12				
9.50	0.50	0.07	0.13				
9.75	0.53	0.08	0.16				
10.00	0.57	0.10	0.18				
10.25	0.61	0.12	0.20				
10.50	0.65	0.14	0.24				
10.75	0.70	0.17	0.28				
11.00	0.75	0.20	0.33				
11.25	0.81	0.24	0.39				
11.50	0.89	0.29	0.53				
11.75	1.07	0.41	0.92				
12.00	1.50	0.74	2.29				
12.25	1.93	1.11	4.95				
12.50	2.11	1.26	2.60				
12.75	2.19	1.33	1.16				
13.00	2.25	1.38	0.76				
13.25	2.30	1.43	0.59				
13.50	2.35	1.48	0.52				
13.75	2.39	1.51	0.48				
14.00	2.43	1.55	0.43				
14.25	2.47	1.58	0.39				
14.50	2.50	1.61	0.36				
14.75	2.53	1.64	0.34				
15.00	2.56	1.67	0.32				
15.25	2.59	1.69	0.30				
15.50	2.61	1.72	0.27				
15.75	2.64	1.74	0.25				
16.00	2.66	1.75	0.23				
16.25	2.68	1.77	0.21				
16.50	2.70	1.79	0.20				
16.75	2.71	1.80	0.19				
17.00	2.73	1.82	0.18				
17.25	2.74	1.83	0.17				
17.50	2.76	1.85	0.16				

357 Electric Avenue - Pre-development

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2 Year Storm
Type III 24-hr Rainfall=3.00"

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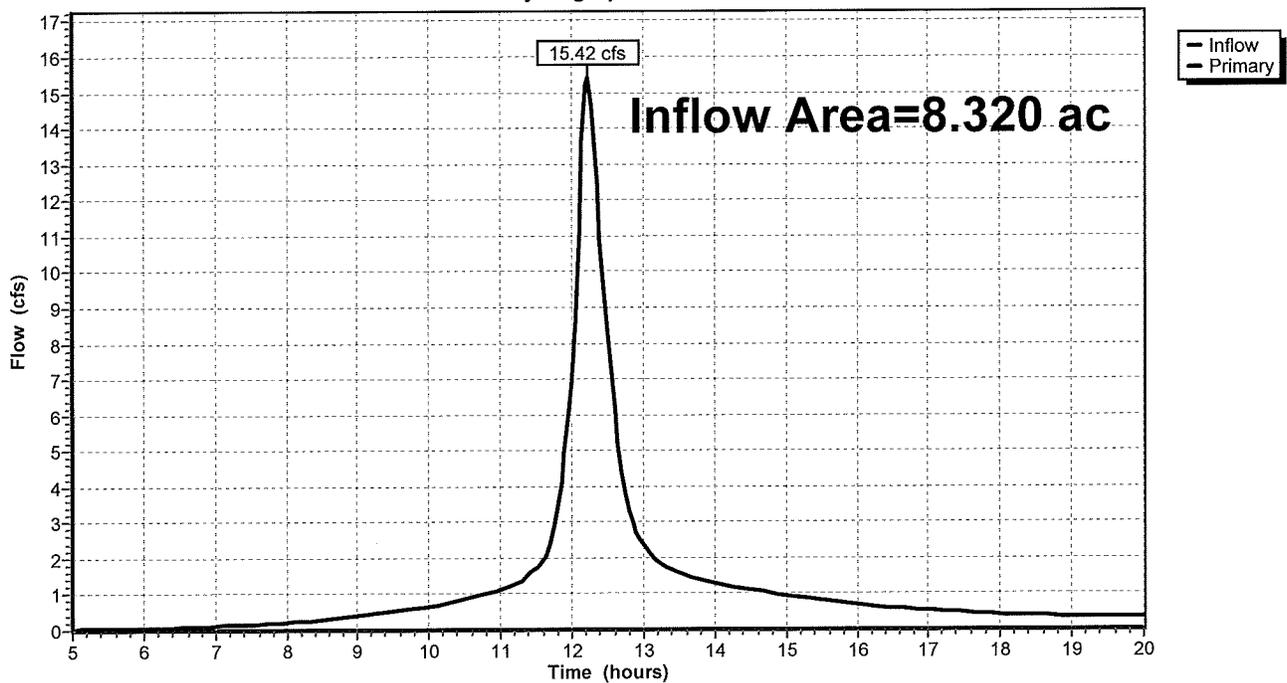
Link DP1: Design Point 1

Inflow Area = 8.320 ac, Inflow Depth > 2.12"
Inflow = 15.42 cfs @ 12.22 hrs, Volume= 1.471 af
Primary = 15.42 cfs @ 12.22 hrs, Volume= 1.471 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link DP1: Design Point 1

Hydrograph



357 Electric Avenue - Pre-development

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2 Year Storm
Type III 24-hr Rainfall=3.00"

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Hydrograph for Link DP1: Design Point 1

Time (hours)	Inflow (cfs)	Elevation (feet)	Primary (cfs)	Time (hours)	Inflow (cfs)	Elevation (feet)	Primary (cfs)
5.00	0.02	0.00	0.02	17.75	0.44	0.00	0.44
5.25	0.03	0.00	0.03	18.00	0.41	0.00	0.41
5.50	0.04	0.00	0.04	18.25	0.39	0.00	0.39
5.75	0.04	0.00	0.04	18.50	0.37	0.00	0.37
6.00	0.05	0.00	0.05	18.75	0.36	0.00	0.36
6.25	0.07	0.00	0.07	19.00	0.36	0.00	0.36
6.50	0.08	0.00	0.08	19.25	0.35	0.00	0.35
6.75	0.10	0.00	0.10	19.50	0.34	0.00	0.34
7.00	0.12	0.00	0.12	19.75	0.33	0.00	0.33
7.25	0.14	0.00	0.14	20.00	0.32	0.00	0.32
7.50	0.16	0.00	0.16				
7.75	0.19	0.00	0.19				
8.00	0.21	0.00	0.21				
8.25	0.24	0.00	0.24				
8.50	0.29	0.00	0.29				
8.75	0.34	0.00	0.34				
9.00	0.39	0.00	0.39				
9.25	0.45	0.00	0.45				
9.50	0.51	0.00	0.51				
9.75	0.58	0.00	0.58				
10.00	0.65	0.00	0.65				
10.25	0.73	0.00	0.73				
10.50	0.85	0.00	0.85				
10.75	0.98	0.00	0.98				
11.00	1.12	0.00	1.12				
11.25	1.31	0.00	1.31				
11.50	1.72	0.00	1.72				
11.75	2.85	0.00	2.85				
12.00	6.91	0.00	6.91				
12.25	15.21	0.00	15.21				
12.50	8.39	0.00	8.39				
12.75	3.79	0.00	3.79				
13.00	2.36	0.00	2.36				
13.25	1.80	0.00	1.80				
13.50	1.58	0.00	1.58				
13.75	1.44	0.00	1.44				
14.00	1.30	0.00	1.30				
14.25	1.17	0.00	1.17				
14.50	1.09	0.00	1.09				
14.75	1.03	0.00	1.03				
15.00	0.96	0.00	0.96				
15.25	0.89	0.00	0.89				
15.50	0.82	0.00	0.82				
15.75	0.76	0.00	0.76				
16.00	0.69	0.00	0.69				
16.25	0.63	0.00	0.63				
16.50	0.59	0.00	0.59				
16.75	0.56	0.00	0.56				
17.00	0.53	0.00	0.53				
17.25	0.50	0.00	0.50				
17.50	0.47	0.00	0.47				

EXISTING CONDITIONS

10 YEAR STORM

357 Electric Avenue - Pre-development

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10 Year Storm
Type III 24-hr Rainfall=4.50"

Page 1

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Overland runoff to southwest

Runoff Area=5.450 ac Runoff Depth>3.59"
Flow Length=708' Tc=17.5 min CN=94 Runoff=16.31 cfs 1.633 af

Subcatchment 2S: Overland runoff to south

Runoff Area=2.870 ac Runoff Depth>3.30"
Flow Length=498' Tc=15.6 min CN=91 Runoff=8.48 cfs 0.789 af

Link DP1: Design Point 1

Inflow=24.76 cfs 2.421 af
Primary=24.76 cfs 2.421 af

Total Runoff Area = 8.320 ac Runoff Volume = 2.421 af Average Runoff Depth = 3.49"

357 Electric Avenue - Pre-development

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10 Year Storm
Type III 24-hr Rainfall=4.50"

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Subcatchment 1S: Overland runoff to southwest

Runoff = 16.31 cfs @ 12.23 hrs, Volume= 1.633 af, Depth> 3.59"

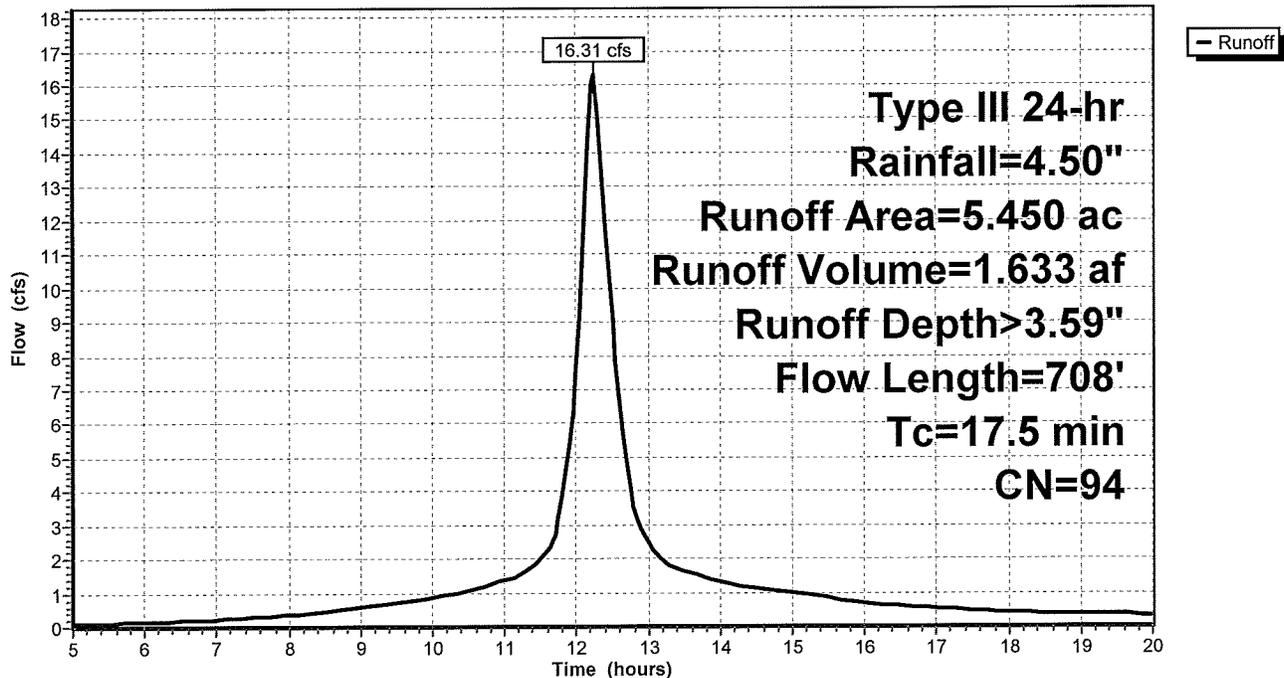
Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr Rainfall=4.50"

Area (ac)	CN	Description
4.610	98	Paved parking & roofs
0.840	70	Woods, Good, HSG C
5.450	94	Weighted Average

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.7	50	0.0200	0.1		Sheet Flow, Path 1 Woods: Light underbrush n= 0.400 P2= 3.00"
3.4	588	0.0200	2.9		Shallow Concentrated Flow, Path 2 Paved Kv= 20.3 fps
1.4	70	0.0285	0.8		Shallow Concentrated Flow, Path 3 Woodland Kv= 5.0 fps
17.5	708	Total			

Subcatchment 1S: Overland runoff to southwest

Hydrograph



357 Electric Avenue - Pre-development

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10 Year Storm
Type III 24-hr Rainfall=4.50"

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Hydrograph for Subcatchment 1S: Overland runoff to southwest

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)	Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
5.00	0.26	0.02	0.10	17.75	4.16	3.48	0.45
5.25	0.27	0.03	0.11	18.00	4.18	3.50	0.42
5.50	0.29	0.03	0.12	18.25	4.19	3.51	0.39
5.75	0.31	0.04	0.14	18.50	4.21	3.53	0.38
6.00	0.32	0.05	0.15	18.75	4.23	3.55	0.37
6.25	0.34	0.05	0.16	19.00	4.24	3.56	0.36
6.50	0.36	0.06	0.19	19.25	4.26	3.58	0.35
6.75	0.38	0.07	0.21	19.50	4.28	3.60	0.34
7.00	0.41	0.09	0.24	19.75	4.29	3.61	0.33
7.25	0.43	0.10	0.27	20.00	4.31	3.63	0.33
7.50	0.46	0.11	0.30				
7.75	0.48	0.13	0.33				
8.00	0.51	0.15	0.36				
8.25	0.54	0.16	0.39				
8.50	0.58	0.19	0.45				
8.75	0.62	0.21	0.51				
9.00	0.66	0.24	0.58				
9.25	0.70	0.27	0.64				
9.50	0.75	0.30	0.72				
9.75	0.80	0.34	0.79				
10.00	0.85	0.38	0.86				
10.25	0.91	0.43	0.95				
10.50	0.97	0.48	1.08				
10.75	1.05	0.54	1.22				
11.00	1.13	0.61	1.37				
11.25	1.22	0.69	1.57				
11.50	1.34	0.80	2.02				
11.75	1.60	1.03	3.22				
12.00	2.25	1.63	7.51				
12.25	2.90	2.25	16.22				
12.50	3.16	2.50	9.03				
12.75	3.28	2.62	4.08				
13.00	3.37	2.71	2.48				
13.25	3.45	2.79	1.87				
13.50	3.53	2.86	1.64				
13.75	3.59	2.92	1.48				
14.00	3.65	2.98	1.34				
14.25	3.70	3.03	1.21				
14.50	3.75	3.08	1.12				
14.75	3.80	3.13	1.05				
15.00	3.84	3.17	0.98				
15.25	3.88	3.21	0.91				
15.50	3.92	3.25	0.84				
15.75	3.96	3.28	0.77				
16.00	3.99	3.31	0.71				
16.25	4.02	3.34	0.64				
16.50	4.04	3.37	0.60				
16.75	4.07	3.39	0.57				
17.00	4.09	3.42	0.54				
17.25	4.12	3.44	0.51				
17.50	4.14	3.46	0.48				

357 Electric Avenue - Pre-development

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10 Year Storm
Type III 24-hr Rainfall=4.50"

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Subcatchment 2S: Overland runoff to south

Runoff = 8.48 cfs @ 12.21 hrs, Volume= 0.789 af, Depth> 3.30"

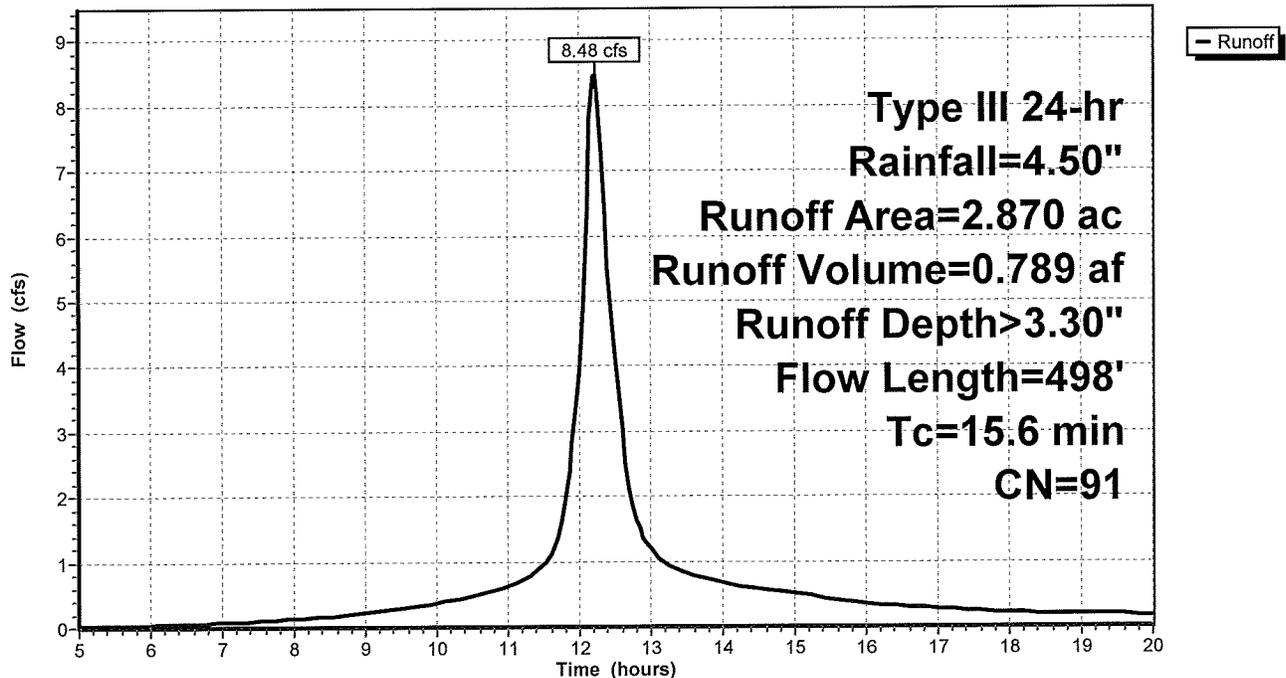
Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr Rainfall=4.50"

Area (ac)	CN	Description
2.200	98	Paved parking & roofs
0.670	70	Woods, Good, HSG C
2.870	91	Weighted Average

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.7	50	0.0200	0.1		Sheet Flow, Path 1 Woods: Light underbrush n= 0.400 P2= 3.00"
2.3	408	0.0220	3.0		Shallow Concentrated Flow, Path 2 Paved Kv= 20.3 fps
0.6	40	0.0500	1.1		Shallow Concentrated Flow, Path 3 Woodland Kv= 5.0 fps
15.6	498	Total			

Subcatchment 2S: Overland runoff to south

Hydrograph



357 Electric Avenue - Pre-development

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10 Year Storm
Type III 24-hr Rainfall=4.50"

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Hydrograph for Subcatchment 2S: Overland runoff to south

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)	Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
5.00	0.26	0.00	0.01	17.75	4.16	3.17	0.23
5.25	0.27	0.01	0.02	18.00	4.18	3.19	0.22
5.50	0.29	0.01	0.03	18.25	4.19	3.20	0.20
5.75	0.31	0.01	0.03	18.50	4.21	3.22	0.20
6.00	0.32	0.01	0.04	18.75	4.23	3.24	0.19
6.25	0.34	0.02	0.05	19.00	4.24	3.25	0.19
6.50	0.36	0.02	0.05	19.25	4.26	3.27	0.18
6.75	0.38	0.03	0.06	19.50	4.28	3.28	0.18
7.00	0.41	0.04	0.08	19.75	4.29	3.30	0.17
7.25	0.43	0.04	0.09	20.00	4.31	3.31	0.17
7.50	0.46	0.05	0.10				
7.75	0.48	0.06	0.11				
8.00	0.51	0.08	0.13				
8.25	0.54	0.09	0.15				
8.50	0.58	0.11	0.17				
8.75	0.62	0.12	0.20				
9.00	0.66	0.15	0.23				
9.25	0.70	0.17	0.26				
9.50	0.75	0.20	0.30				
9.75	0.80	0.23	0.33				
10.00	0.85	0.26	0.37				
10.25	0.91	0.30	0.41				
10.50	0.97	0.34	0.48				
10.75	1.05	0.39	0.55				
11.00	1.13	0.45	0.62				
11.25	1.22	0.52	0.73				
11.50	1.34	0.61	0.96				
11.75	1.60	0.82	1.64				
12.00	2.25	1.38	3.93				
12.25	2.90	1.98	8.15				
12.50	3.16	2.22	4.20				
12.75	3.28	2.33	1.85				
13.00	3.37	2.42	1.20				
13.25	3.45	2.50	0.93				
13.50	3.53	2.57	0.83				
13.75	3.59	2.63	0.75				
14.00	3.65	2.68	0.68				
14.25	3.70	2.73	0.61				
14.50	3.75	2.78	0.57				
14.75	3.80	2.83	0.54				
15.00	3.84	2.87	0.50				
15.25	3.88	2.91	0.46				
15.50	3.92	2.94	0.43				
15.75	3.96	2.98	0.39				
16.00	3.99	3.00	0.36				
16.25	4.02	3.03	0.33				
16.50	4.04	3.06	0.31				
16.75	4.07	3.08	0.29				
17.00	4.09	3.11	0.28				
17.25	4.12	3.13	0.26				
17.50	4.14	3.15	0.25				

357 Electric Avenue - Pre-development

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10 Year Storm
Type III 24-hr Rainfall=4.50"

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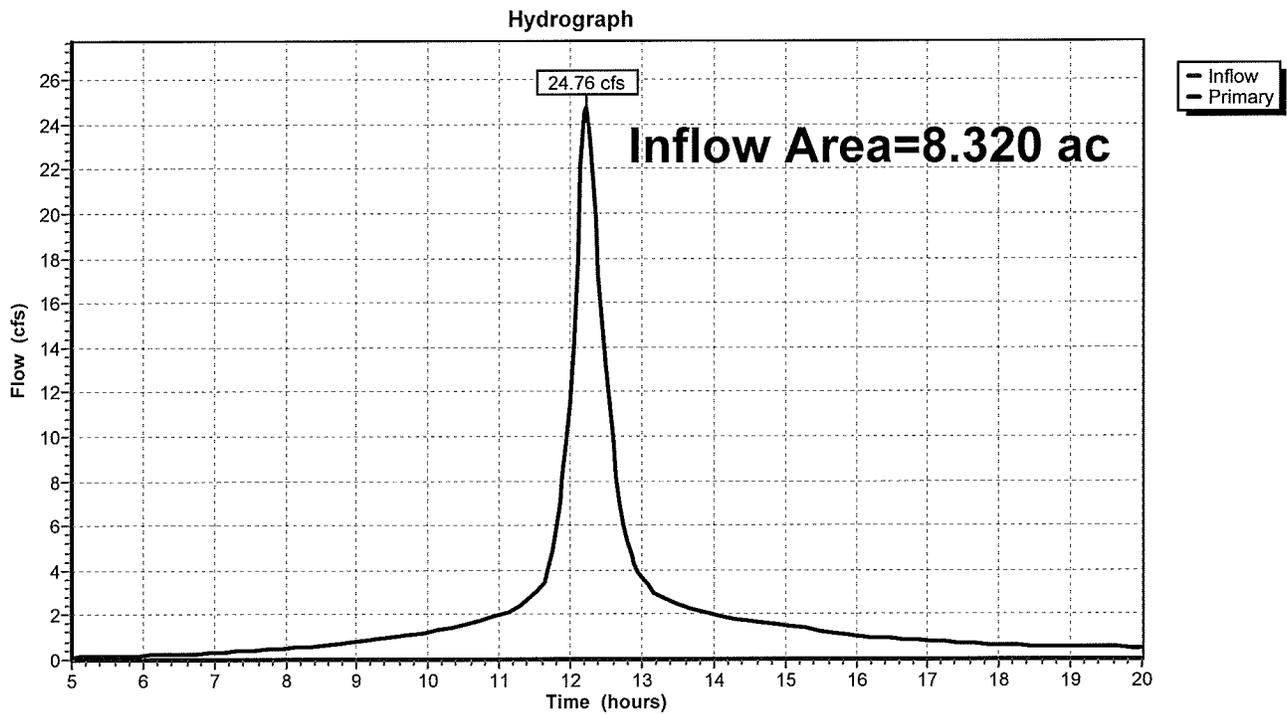
11/20/2015

Link DP1: Design Point 1

Inflow Area = 8.320 ac, Inflow Depth > 3.49"
Inflow = 24.76 cfs @ 12.22 hrs, Volume= 2.421 af
Primary = 24.76 cfs @ 12.22 hrs, Volume= 2.421 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link DP1: Design Point 1



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10 Year Storm
Type III 24-hr Rainfall=4.50"

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Hydrograph for Link DP1: Design Point 1

Time (hours)	Inflow (cfs)	Elevation (feet)	Primary (cfs)	Time (hours)	Inflow (cfs)	Elevation (feet)	Primary (cfs)
5.00	0.11	0.00	0.11	17.75	0.68	0.00	0.68
5.25	0.13	0.00	0.13	18.00	0.64	0.00	0.64
5.50	0.15	0.00	0.15	18.25	0.60	0.00	0.60
5.75	0.17	0.00	0.17	18.50	0.58	0.00	0.58
6.00	0.19	0.00	0.19	18.75	0.56	0.00	0.56
6.25	0.21	0.00	0.21	19.00	0.55	0.00	0.55
6.50	0.24	0.00	0.24	19.25	0.53	0.00	0.53
6.75	0.28	0.00	0.28	19.50	0.52	0.00	0.52
7.00	0.31	0.00	0.31	19.75	0.51	0.00	0.51
7.25	0.35	0.00	0.35	20.00	0.49	0.00	0.49
7.50	0.40	0.00	0.40				
7.75	0.44	0.00	0.44				
8.00	0.49	0.00	0.49				
8.25	0.54	0.00	0.54				
8.50	0.62	0.00	0.62				
8.75	0.71	0.00	0.71				
9.00	0.81	0.00	0.81				
9.25	0.91	0.00	0.91				
9.50	1.01	0.00	1.01				
9.75	1.12	0.00	1.12				
10.00	1.23	0.00	1.23				
10.25	1.36	0.00	1.36				
10.50	1.55	0.00	1.55				
10.75	1.77	0.00	1.77				
11.00	1.99	0.00	1.99				
11.25	2.31	0.00	2.31				
11.50	2.98	0.00	2.98				
11.75	4.85	0.00	4.85				
12.00	11.45	0.00	11.45				
12.25	24.38	0.00	24.38				
12.50	13.23	0.00	13.23				
12.75	5.93	0.00	5.93				
13.00	3.68	0.00	3.68				
13.25	2.80	0.00	2.80				
13.50	2.46	0.00	2.46				
13.75	2.23	0.00	2.23				
14.00	2.02	0.00	2.02				
14.25	1.82	0.00	1.82				
14.50	1.69	0.00	1.69				
14.75	1.59	0.00	1.59				
15.00	1.48	0.00	1.48				
15.25	1.38	0.00	1.38				
15.50	1.27	0.00	1.27				
15.75	1.17	0.00	1.17				
16.00	1.06	0.00	1.06				
16.25	0.97	0.00	0.97				
16.50	0.91	0.00	0.91				
16.75	0.86	0.00	0.86				
17.00	0.82	0.00	0.82				
17.25	0.77	0.00	0.77				
17.50	0.73	0.00	0.73				

EXISTING CONDITIONS

25 YEAR STORM

357 Electric Avenue - Pre-development

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25 Year Storm
Type III 24-hr Rainfall=5.30"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Overland runoff to southwest

Runoff Area=5.450 ac Runoff Depth>4.33"
Flow Length=708' Tc=17.5 min CN=94 Runoff=19.48 cfs 1.968 af

Subcatchment 2S: Overland runoff to south

Runoff Area=2.870 ac Runoff Depth>4.03"
Flow Length=498' Tc=15.6 min CN=91 Runoff=10.25 cfs 0.964 af

Link DP1: Design Point 1

Inflow=29.71 cfs 2.932 af
Primary=29.71 cfs 2.932 af

Total Runoff Area = 8.320 ac Runoff Volume = 2.932 af Average Runoff Depth = 4.23"

357 Electric Avenue - Pre-development

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25 Year Storm
Type III 24-hr Rainfall=5.30"

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Subcatchment 1S: Overland runoff to southwest

Runoff = 19.48 cfs @ 12.23 hrs, Volume= 1.968 af, Depth> 4.33"

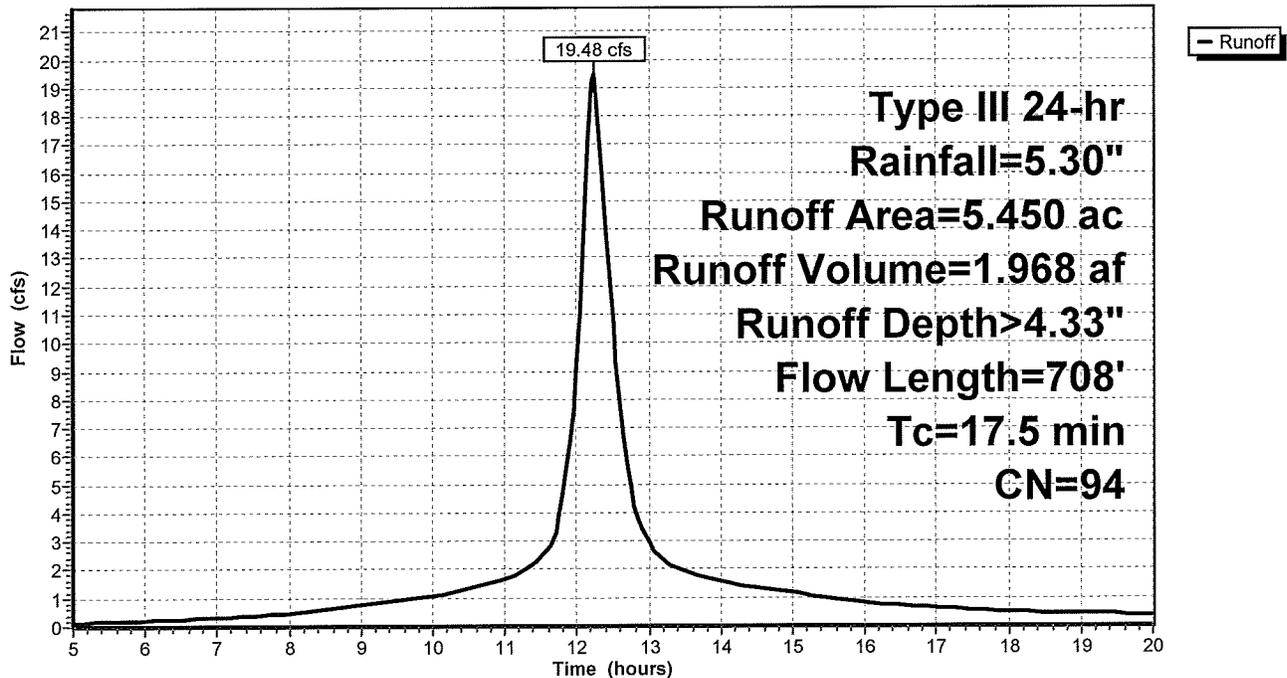
Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr Rainfall=5.30"

Area (ac)	CN	Description
4.610	98	Paved parking & roofs
0.840	70	Woods, Good, HSG C
5.450	94	Weighted Average

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.7	50	0.0200	0.1		Sheet Flow, Path 1 Woods: Light underbrush n= 0.400 P2= 3.00"
3.4	588	0.0200	2.9		Shallow Concentrated Flow, Path 2 Paved Kv= 20.3 fps
1.4	70	0.0285	0.8		Shallow Concentrated Flow, Path 3 Woodland Kv= 5.0 fps
17.5	708	Total			

Subcatchment 1S: Overland runoff to southwest

Hydrograph



357 Electric Avenue - Pre-development

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25 Year Storm
Type III 24-hr Rainfall=5.30"

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Hydrograph for Subcatchment 1S: Overland runoff to southwest

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)	Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
5.00	0.30	0.04	0.14	17.75	4.90	4.21	0.53
5.25	0.32	0.04	0.16	18.00	4.92	4.23	0.50
5.50	0.34	0.05	0.18	18.25	4.94	4.25	0.47
5.75	0.36	0.06	0.19	18.50	4.96	4.27	0.45
6.00	0.38	0.07	0.21	18.75	4.98	4.29	0.44
6.25	0.40	0.08	0.23	19.00	5.00	4.31	0.43
6.50	0.43	0.10	0.26	19.25	5.02	4.33	0.42
6.75	0.45	0.11	0.29	19.50	5.04	4.34	0.41
7.00	0.48	0.13	0.32	19.75	5.05	4.36	0.40
7.25	0.51	0.14	0.35	20.00	5.07	4.38	0.39
7.50	0.54	0.16	0.39				
7.75	0.57	0.18	0.43				
8.00	0.60	0.20	0.47				
8.25	0.64	0.23	0.51				
8.50	0.68	0.26	0.58				
8.75	0.72	0.29	0.65				
9.00	0.77	0.32	0.73				
9.25	0.82	0.36	0.82				
9.50	0.88	0.41	0.90				
9.75	0.94	0.45	0.99				
10.00	1.00	0.51	1.08				
10.25	1.07	0.56	1.18				
10.50	1.15	0.63	1.33				
10.75	1.23	0.70	1.50				
11.00	1.32	0.78	1.68				
11.25	1.44	0.88	1.92				
11.50	1.58	1.01	2.46				
11.75	1.88	1.29	3.89				
12.00	2.65	2.01	9.04				
12.25	3.42	2.75	19.37				
12.50	3.72	3.05	10.75				
12.75	3.86	3.19	4.84				
13.00	3.97	3.30	2.94				
13.25	4.07	3.39	2.22				
13.50	4.15	3.47	1.94				
13.75	4.23	3.55	1.76				
14.00	4.30	3.62	1.59				
14.25	4.36	3.68	1.43				
14.50	4.42	3.74	1.33				
14.75	4.48	3.79	1.25				
15.00	4.53	3.84	1.16				
15.25	4.58	3.89	1.08				
15.50	4.62	3.93	1.00				
15.75	4.66	3.97	0.92				
16.00	4.70	4.01	0.83				
16.25	4.73	4.04	0.76				
16.50	4.76	4.07	0.71				
16.75	4.79	4.10	0.68				
17.00	4.82	4.13	0.64				
17.25	4.85	4.16	0.61				
17.50	4.87	4.18	0.57				

357 Electric Avenue - Pre-development

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25 Year Storm
Type III 24-hr Rainfall=5.30"

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Subcatchment 2S: Overland runoff to south

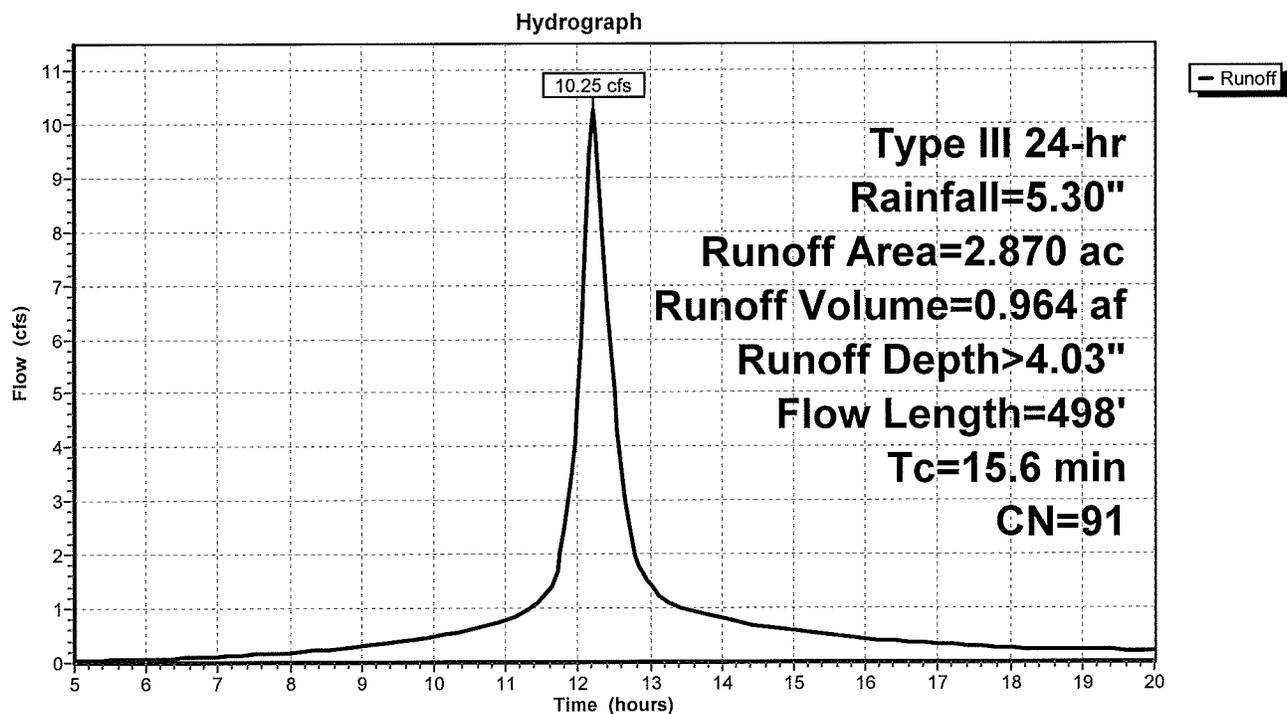
Runoff = 10.25 cfs @ 12.21 hrs, Volume= 0.964 af, Depth> 4.03"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr Rainfall=5.30"

Area (ac)	CN	Description
2.200	98	Paved parking & roofs
0.670	70	Woods, Good, HSG C
2.870	91	Weighted Average

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.7	50	0.0200	0.1		Sheet Flow, Path 1 Woods: Light underbrush n= 0.400 P2= 3.00"
2.3	408	0.0220	3.0		Shallow Concentrated Flow, Path 2 Paved Kv= 20.3 fps
0.6	40	0.0500	1.1		Shallow Concentrated Flow, Path 3 Woodland Kv= 5.0 fps
15.6	498	Total			

Subcatchment 2S: Overland runoff to south



357 Electric Avenue - Pre-development

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25 Year Storm
Type III 24-hr Rainfall=5.30"

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Hydrograph for Subcatchment 2S: Overland runoff to south

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)	Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
5.00	0.30	0.01	0.03	17.75	4.90	3.88	0.27
5.25	0.32	0.01	0.04	18.00	4.92	3.90	0.26
5.50	0.34	0.02	0.05	18.25	4.94	3.92	0.24
5.75	0.36	0.02	0.06	18.50	4.96	3.94	0.23
6.00	0.38	0.03	0.06	18.75	4.98	3.96	0.23
6.25	0.40	0.04	0.07	19.00	5.00	3.98	0.22
6.50	0.43	0.04	0.08	19.25	5.02	4.00	0.22
6.75	0.45	0.05	0.10	19.50	5.04	4.02	0.21
7.00	0.48	0.06	0.11	19.75	5.05	4.04	0.20
7.25	0.51	0.07	0.13	20.00	5.07	4.05	0.20
7.50	0.54	0.09	0.14				
7.75	0.57	0.10	0.16				
8.00	0.60	0.12	0.18				
8.25	0.64	0.14	0.20				
8.50	0.68	0.16	0.23				
8.75	0.72	0.18	0.27				
9.00	0.77	0.21	0.31				
9.25	0.82	0.24	0.35				
9.50	0.88	0.28	0.39				
9.75	0.94	0.32	0.43				
10.00	1.00	0.36	0.48				
10.25	1.07	0.41	0.53				
10.50	1.15	0.47	0.61				
10.75	1.23	0.53	0.70				
11.00	1.32	0.60	0.79				
11.25	1.44	0.69	0.92				
11.50	1.58	0.81	1.20				
11.75	1.88	1.06	2.02				
12.00	2.65	1.75	4.81				
12.25	3.42	2.46	9.85				
12.50	3.72	2.75	5.04				
12.75	3.86	2.89	2.22				
13.00	3.97	2.99	1.43				
13.25	4.07	3.08	1.11				
13.50	4.15	3.16	0.99				
13.75	4.23	3.24	0.90				
14.00	4.30	3.30	0.81				
14.25	4.36	3.36	0.73				
14.50	4.42	3.42	0.68				
14.75	4.48	3.47	0.64				
15.00	4.53	3.52	0.60				
15.25	4.58	3.57	0.55				
15.50	4.62	3.61	0.51				
15.75	4.66	3.65	0.47				
16.00	4.70	3.69	0.43				
16.25	4.73	3.72	0.39				
16.50	4.76	3.75	0.37				
16.75	4.79	3.78	0.35				
17.00	4.82	3.81	0.33				
17.25	4.85	3.83	0.31				
17.50	4.87	3.86	0.29				

357 Electric Avenue - Pre-development

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25 Year Storm
Type III 24-hr Rainfall=5.30"

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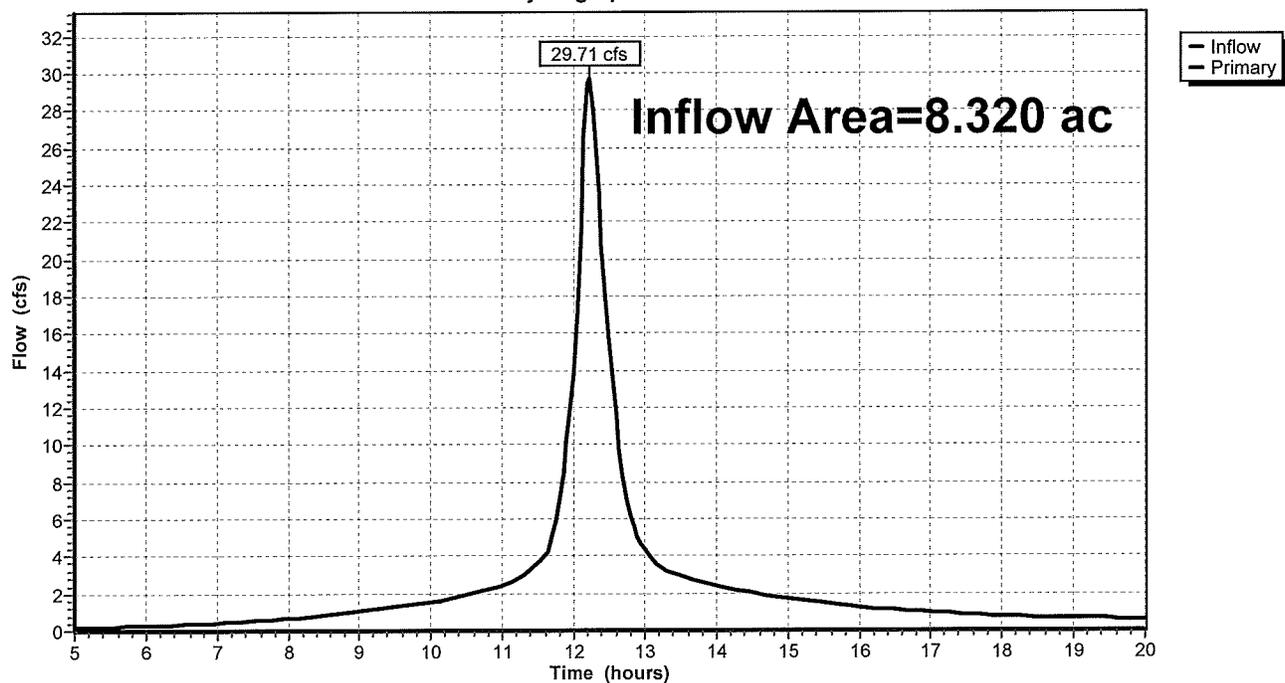
Link DP1: Design Point 1

Inflow Area = 8.320 ac, Inflow Depth > 4.23"
Inflow = 29.71 cfs @ 12.22 hrs, Volume= 2.932 af
Primary = 29.71 cfs @ 12.22 hrs, Volume= 2.932 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link DP1: Design Point 1

Hydrograph



357 Electric Avenue - Pre-development

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25 Year Storm
Type III 24-hr Rainfall=5.30"

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Hydrograph for Link DP1: Design Point 1

Time (hours)	Inflow (cfs)	Elevation (feet)	Primary (cfs)	Time (hours)	Inflow (cfs)	Elevation (feet)	Primary (cfs)
5.00	0.18	0.00	0.18	17.75	0.81	0.00	0.81
5.25	0.20	0.00	0.20	18.00	0.75	0.00	0.75
5.50	0.22	0.00	0.22	18.25	0.71	0.00	0.71
5.75	0.25	0.00	0.25	18.50	0.68	0.00	0.68
6.00	0.27	0.00	0.27	18.75	0.67	0.00	0.67
6.25	0.30	0.00	0.30	19.00	0.65	0.00	0.65
6.50	0.34	0.00	0.34	19.25	0.63	0.00	0.63
6.75	0.38	0.00	0.38	19.50	0.62	0.00	0.62
7.00	0.43	0.00	0.43	19.75	0.60	0.00	0.60
7.25	0.48	0.00	0.48	20.00	0.58	0.00	0.58
7.50	0.54	0.00	0.54				
7.75	0.59	0.00	0.59				
8.00	0.65	0.00	0.65				
8.25	0.71	0.00	0.71				
8.50	0.81	0.00	0.81				
8.75	0.92	0.00	0.92				
9.00	1.04	0.00	1.04				
9.25	1.16	0.00	1.16				
9.50	1.29	0.00	1.29				
9.75	1.42	0.00	1.42				
10.00	1.55	0.00	1.55				
10.25	1.71	0.00	1.71				
10.50	1.94	0.00	1.94				
10.75	2.20	0.00	2.20				
11.00	2.46	0.00	2.46				
11.25	2.84	0.00	2.84				
11.50	3.66	0.00	3.66				
11.75	5.92	0.00	5.92				
12.00	13.86	0.00	13.86				
12.25	29.22	0.00	29.22				
12.50	15.79	0.00	15.79				
12.75	7.06	0.00	7.06				
13.00	4.37	0.00	4.37				
13.25	3.33	0.00	3.33				
13.50	2.93	0.00	2.93				
13.75	2.65	0.00	2.65				
14.00	2.39	0.00	2.39				
14.25	2.16	0.00	2.16				
14.50	2.01	0.00	2.01				
14.75	1.88	0.00	1.88				
15.00	1.76	0.00	1.76				
15.25	1.64	0.00	1.64				
15.50	1.51	0.00	1.51				
15.75	1.39	0.00	1.39				
16.00	1.26	0.00	1.26				
16.25	1.15	0.00	1.15				
16.50	1.08	0.00	1.08				
16.75	1.03	0.00	1.03				
17.00	0.97	0.00	0.97				
17.25	0.92	0.00	0.92				
17.50	0.86	0.00	0.86				

EXISTING CONDITIONS

100 YEAR STORM

357 Electric Avenue - Pre-development

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100 Year Storm
Type III 24-hr Rainfall=6.60"

Page 1

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Overland runoff to southwest

Runoff Area=5.450 ac Runoff Depth>5.53"
Flow Length=708' Tc=17.5 min CN=94 Runoff=24.60 cfs 2.513 af

Subcatchment 2S: Overland runoff to south

Runoff Area=2.870 ac Runoff Depth>5.23"
Flow Length=498' Tc=15.6 min CN=91 Runoff=13.12 cfs 1.250 af

Link DP1: Design Point 1

Inflow=37.68 cfs 3.763 af
Primary=37.68 cfs 3.763 af

Total Runoff Area = 8.320 ac Runoff Volume = 3.763 af Average Runoff Depth = 5.43"

357 Electric Avenue - Pre-development

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100 Year Storm
Type III 24-hr Rainfall=6.60"

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Subcatchment 1S: Overland runoff to southwest

Runoff = 24.60 cfs @ 12.23 hrs, Volume= 2.513 af, Depth> 5.53"

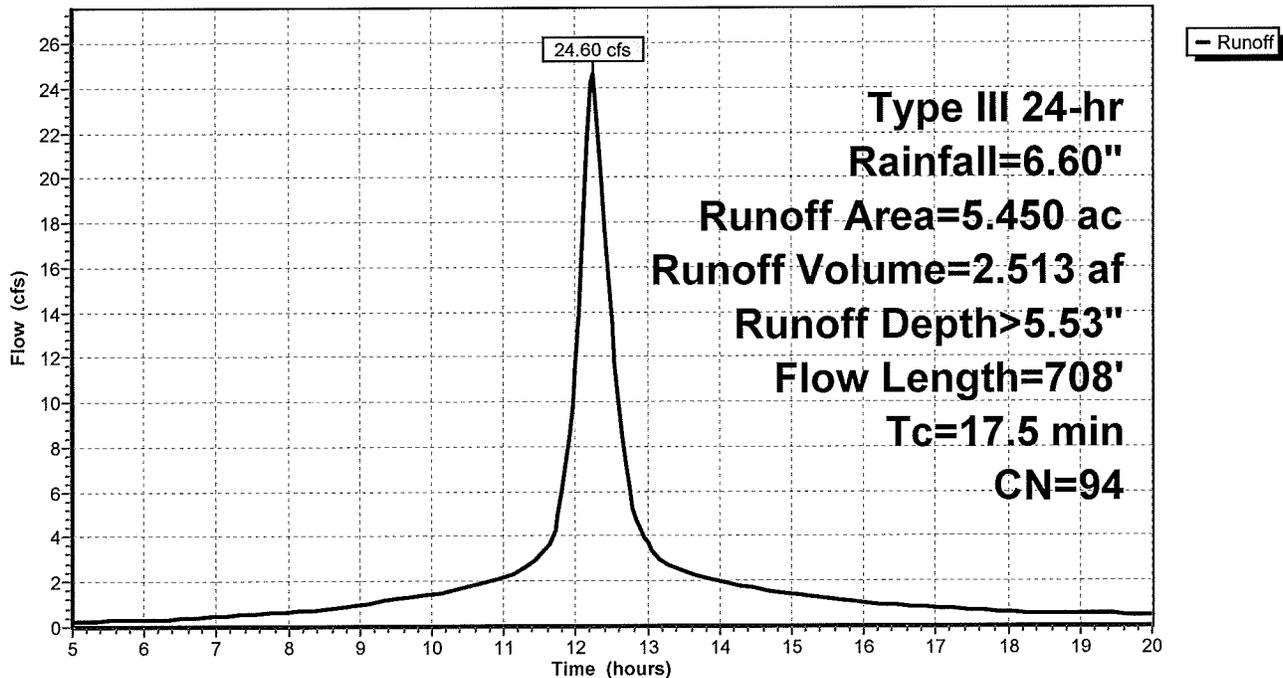
Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr Rainfall=6.60"

Area (ac)	CN	Description
4.610	98	Paved parking & roofs
0.840	70	Woods, Good, HSG C
5.450	94	Weighted Average

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.7	50	0.0200	0.1		Sheet Flow, Path 1 Woods: Light underbrush n= 0.400 P2= 3.00"
3.4	588	0.0200	2.9		Shallow Concentrated Flow, Path 2 Paved Kv= 20.3 fps
1.4	70	0.0285	0.8		Shallow Concentrated Flow, Path 3 Woodland Kv= 5.0 fps
17.5	708	Total			

Subcatchment 1S: Overland runoff to southwest

Hydrograph



357 Electric Avenue - Pre-development

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100 Year Storm
Type III 24-hr Rainfall=6.60"

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Hydrograph for Subcatchment 1S: Overland runoff to southwest

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)	Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
5.00	0.37	0.07	0.23	17.75	6.10	5.39	0.67
5.25	0.40	0.08	0.25	18.00	6.12	5.42	0.62
5.50	0.42	0.09	0.27	18.25	6.15	5.45	0.58
5.75	0.45	0.11	0.29	18.50	6.18	5.47	0.56
6.00	0.48	0.12	0.31	18.75	6.20	5.50	0.55
6.25	0.50	0.14	0.34	19.00	6.23	5.52	0.54
6.50	0.53	0.16	0.37	19.25	6.25	5.54	0.52
6.75	0.56	0.18	0.42	19.50	6.27	5.57	0.51
7.00	0.60	0.20	0.46	19.75	6.29	5.59	0.50
7.25	0.63	0.22	0.51	20.00	6.32	5.61	0.48
7.50	0.67	0.25	0.55				
7.75	0.71	0.28	0.60				
8.00	0.75	0.31	0.65				
8.25	0.80	0.34	0.71				
8.50	0.85	0.38	0.79				
8.75	0.90	0.42	0.89				
9.00	0.96	0.47	0.99				
9.25	1.03	0.53	1.10				
9.50	1.10	0.58	1.20				
9.75	1.17	0.65	1.31				
10.00	1.25	0.71	1.42				
10.25	1.33	0.79	1.55				
10.50	1.43	0.87	1.74				
10.75	1.53	0.97	1.96				
11.00	1.65	1.07	2.18				
11.25	1.79	1.20	2.48				
11.50	1.97	1.37	3.16				
11.75	2.34	1.72	4.99				
12.00	3.30	2.64	11.51				
12.25	4.26	3.57	24.45				
12.50	4.63	3.95	13.52				
12.75	4.81	4.12	6.08				
13.00	4.95	4.26	3.69				
13.25	5.07	4.37	2.78				
13.50	5.17	4.48	2.43				
13.75	5.27	4.57	2.20				
14.00	5.35	4.66	1.99				
14.25	5.43	4.73	1.79				
14.50	5.50	4.81	1.67				
14.75	5.57	4.87	1.56				
15.00	5.64	4.94	1.46				
15.25	5.70	5.00	1.35				
15.50	5.75	5.05	1.25				
15.75	5.80	5.10	1.15				
16.00	5.85	5.15	1.04				
16.25	5.89	5.19	0.95				
16.50	5.93	5.23	0.89				
16.75	5.97	5.26	0.85				
17.00	6.00	5.30	0.80				
17.25	6.04	5.33	0.76				
17.50	6.07	5.36	0.71				

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100 Year Storm
Type III 24-hr Rainfall=6.60"

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Subcatchment 2S: Overland runoff to south

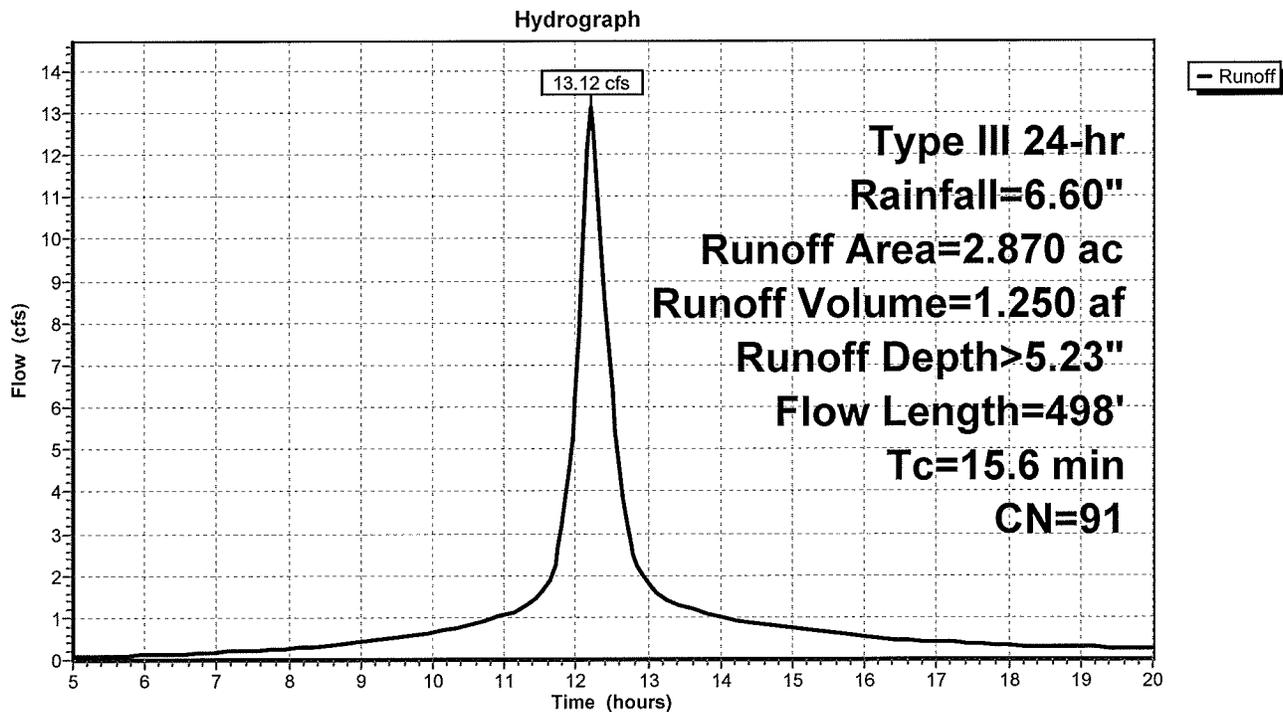
Runoff = 13.12 cfs @ 12.21 hrs, Volume= 1.250 af, Depth> 5.23"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr Rainfall=6.60"

Area (ac)	CN	Description
2.200	98	Paved parking & roofs
0.670	70	Woods, Good, HSG C
2.870	91	Weighted Average

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.7	50	0.0200	0.1		Sheet Flow, Path 1 Woods: Light underbrush n= 0.400 P2= 3.00"
2.3	408	0.0220	3.0		Shallow Concentrated Flow, Path 2 Paved Kv= 20.3 fps
0.6	40	0.0500	1.1		Shallow Concentrated Flow, Path 3 Woodland Kv= 5.0 fps
15.6	498	Total			

Subcatchment 2S: Overland runoff to south



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Hydrograph for Subcatchment 2S: Overland runoff to south

Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)	Time (hours)	Precip. (inches)	Excess (inches)	Runoff (cfs)
5.00	0.37	0.03	0.07	17.75	6.10	5.05	0.35
5.25	0.40	0.03	0.08	18.00	6.12	5.08	0.32
5.50	0.42	0.04	0.09	18.25	6.15	5.10	0.30
5.75	0.45	0.05	0.10	18.50	6.18	5.13	0.29
6.00	0.48	0.06	0.11	18.75	6.20	5.15	0.29
6.25	0.50	0.07	0.12	19.00	6.23	5.18	0.28
6.50	0.53	0.08	0.14	19.25	6.25	5.20	0.27
6.75	0.56	0.10	0.16	19.50	6.27	5.22	0.26
7.00	0.60	0.11	0.18	19.75	6.29	5.25	0.26
7.25	0.63	0.13	0.20	20.00	6.32	5.27	0.25
7.50	0.67	0.15	0.22				
7.75	0.71	0.18	0.24				
8.00	0.75	0.20	0.27				
8.25	0.80	0.23	0.30				
8.50	0.85	0.26	0.34				
8.75	0.90	0.29	0.39				
9.00	0.96	0.33	0.44				
9.25	1.03	0.38	0.49				
9.50	1.10	0.43	0.55				
9.75	1.17	0.48	0.60				
10.00	1.25	0.54	0.66				
10.25	1.33	0.61	0.73				
10.50	1.43	0.68	0.83				
10.75	1.53	0.77	0.94				
11.00	1.65	0.86	1.05				
11.25	1.79	0.98	1.22				
11.50	1.97	1.13	1.58				
11.75	2.34	1.47	2.66				
12.00	3.30	2.35	6.24				
12.25	4.26	3.26	12.58				
12.50	4.63	3.63	6.39				
12.75	4.81	3.80	2.80				
13.00	4.95	3.93	1.81				
13.25	5.07	4.05	1.40				
13.50	5.17	4.15	1.24				
13.75	5.27	4.24	1.13				
14.00	5.35	4.32	1.02				
14.25	5.43	4.40	0.92				
14.50	5.50	4.47	0.86				
14.75	5.57	4.54	0.80				
15.00	5.64	4.60	0.75				
15.25	5.70	4.66	0.70				
15.50	5.75	4.71	0.64				
15.75	5.80	4.76	0.59				
16.00	5.85	4.81	0.54				
16.25	5.89	4.85	0.49				
16.50	5.93	4.89	0.46				
16.75	5.97	4.92	0.44				
17.00	6.00	4.96	0.41				
17.25	6.04	4.99	0.39				
17.50	6.07	5.02	0.37				

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100 Year Storm
Type III 24-hr Rainfall=6.60"

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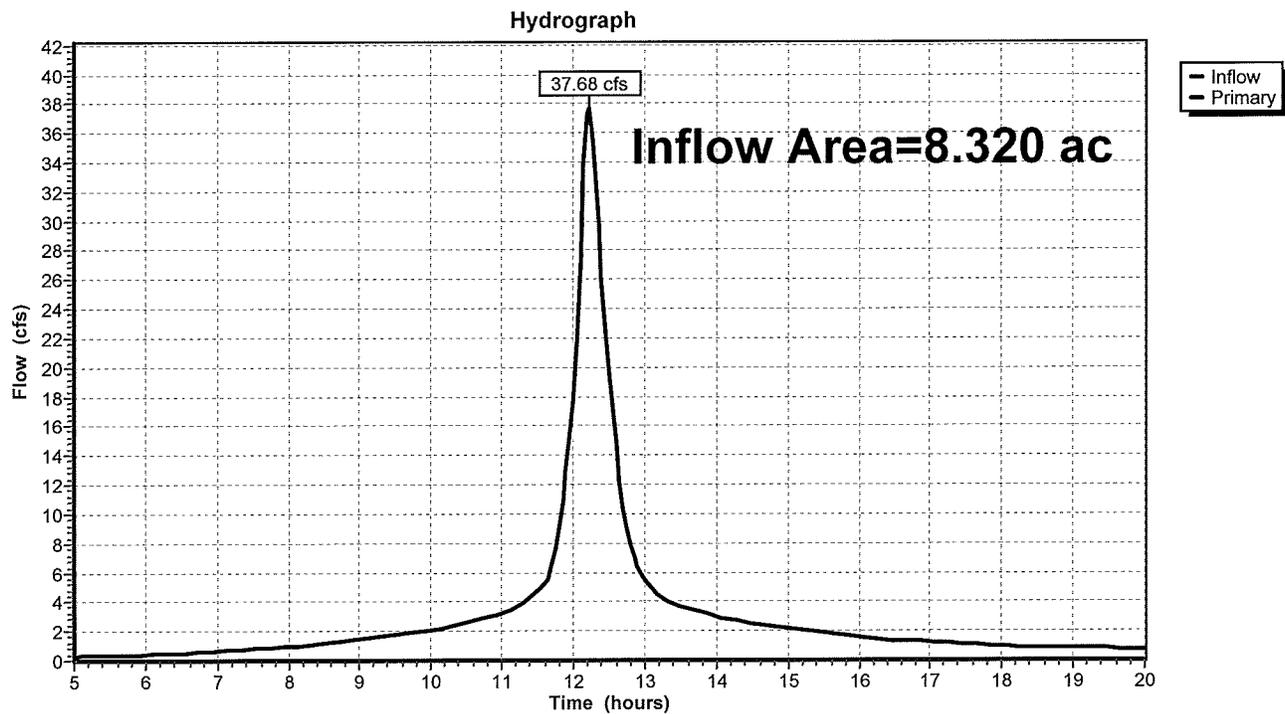
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Link DP1: Design Point 1

Inflow Area = 8.320 ac, Inflow Depth > 5.43"
Inflow = 37.68 cfs @ 12.22 hrs, Volume= 3.763 af
Primary = 37.68 cfs @ 12.22 hrs, Volume= 3.763 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link DP1: Design Point 1



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Type III 24-hr Rainfall=6.60"

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Hydrograph for Link DP1: Design Point 1

Time (hours)	Inflow (cfs)	Elevation (feet)	Primary (cfs)	Time (hours)	Inflow (cfs)	Elevation (feet)	Primary (cfs)
5.00	0.30	0.00	0.30	17.75	1.01	0.00	1.01
5.25	0.33	0.00	0.33	18.00	0.95	0.00	0.95
5.50	0.36	0.00	0.36	18.25	0.89	0.00	0.89
5.75	0.39	0.00	0.39	18.50	0.86	0.00	0.86
6.00	0.42	0.00	0.42	18.75	0.83	0.00	0.83
6.25	0.46	0.00	0.46	19.00	0.81	0.00	0.81
6.50	0.51	0.00	0.51	19.25	0.79	0.00	0.79
6.75	0.57	0.00	0.57	19.50	0.77	0.00	0.77
7.00	0.64	0.00	0.64	19.75	0.75	0.00	0.75
7.25	0.70	0.00	0.70	20.00	0.73	0.00	0.73
7.50	0.77	0.00	0.77				
7.75	0.84	0.00	0.84				
8.00	0.92	0.00	0.92				
8.25	1.00	0.00	1.00				
8.50	1.13	0.00	1.13				
8.75	1.28	0.00	1.28				
9.00	1.43	0.00	1.43				
9.25	1.59	0.00	1.59				
9.50	1.75	0.00	1.75				
9.75	1.91	0.00	1.91				
10.00	2.08	0.00	2.08				
10.25	2.28	0.00	2.28				
10.50	2.57	0.00	2.57				
10.75	2.89	0.00	2.89				
11.00	3.23	0.00	3.23				
11.25	3.71	0.00	3.71				
11.50	4.75	0.00	4.75				
11.75	7.65	0.00	7.65				
12.00	17.74	0.00	17.74				
12.25	37.03	0.00	37.03				
12.50	19.91	0.00	19.91				
12.75	8.88	0.00	8.88				
13.00	5.50	0.00	5.50				
13.25	4.19	0.00	4.19				
13.50	3.68	0.00	3.68				
13.75	3.33	0.00	3.33				
14.00	3.01	0.00	3.01				
14.25	2.71	0.00	2.71				
14.50	2.52	0.00	2.52				
14.75	2.36	0.00	2.36				
15.00	2.21	0.00	2.21				
15.25	2.05	0.00	2.05				
15.50	1.89	0.00	1.89				
15.75	1.74	0.00	1.74				
16.00	1.58	0.00	1.58				
16.25	1.44	0.00	1.44				
16.50	1.36	0.00	1.36				
16.75	1.29	0.00	1.29				
17.00	1.22	0.00	1.22				
17.25	1.15	0.00	1.15				
17.50	1.08	0.00	1.08				